

Boa™ Coil

EXPANSION ANCHORS - NON-CRACKED CONCRETE

GENERAL INFORMATION

| Performance Related | Material | Installation Related |
|---------------------|----------|----------------------|
| | | |

Product

The Boa™ Coil Anchor is a heavy duty, rotation setting expansion anchor.

Benefits, Advantages and Features

High load capacity:

- Expansion coil locks into concrete to give cast-in type performance.
- High tensile capacity of grade 8.8 steel bolt.

High clamping load:

- Rotation setting action pulls down.

Resistant to cyclic loading:

- Pull-down action.

Fast installation:

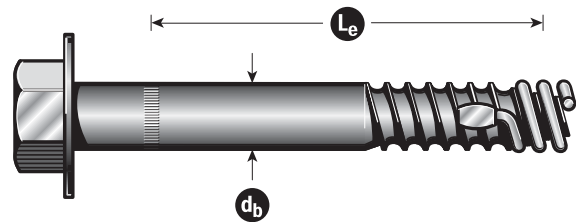
- Through fixing eliminates marking out and repositioning of fixtures.

Easy and fast to remove:

- Expansion coil stays in hole leaving no protruding metal parts to grind off.

Ramset Design Method:

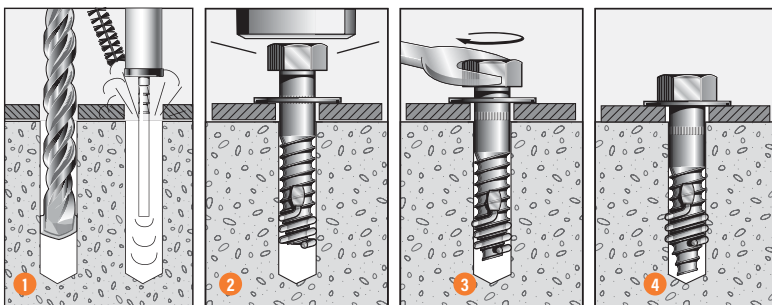
- Uses technical data validated from testing in ANZ concrete substrates



Principal Applications

- Installing handrails and balustrades
- Machinery hold down
- Formwork support
- Safety barriers
- Scaffolding

Installation



1. Drill or core a hole to the recommended diameter and depth using the fixture as a template. Clean the hole thoroughly with a hole cleaning brush. Remove the debris with a hand pump, compressed air, or vacuum.
2. After ensuring that the anchor is assembled correctly (the coil tab points up the anchor), insert the anchor through the fixture.
3. Tap the anchor down to the depth set mark, with a hammer, and stop.
4. Wind the anchor down, with an appropriately sized spanner or socket wrench, until the washer is firmly held to the fixture and stop (5 turns). Ensure washer is tight and snug fit.
5. The Boa™ Coil anchor is ready to take load. (The bolt can be removed leaving the coil in the hole. To re-insert, follow steps 3 - 4.)

Boa™ Coil

EXPANSION ANCHORS - NON-CRACKED CONCRETE

Mechanical Anchoring

Installation and performance details

| Anchor Size, d_b (mm) | Installation details | | | | Optimum dimensions* | | Reduced Characteristic Capacity | | | |
|-------------------------|--------------------------------|-----------------------------------|----------------------------------|---------------------|---------------------------|----------------------------|---------------------------------|--|--------|--------|
| | Drilled Hole diam., d_h (mm) | Fixture hole diameter, d_f (mm) | Anchor effective depth, h (mm) | Turns to set anchor | Edge distance, e_c (mm) | Anchor spacing, a_c (mm) | Steel Shear, ϕV_{us} (kN) | Non-Cracked Concrete Tension, ϕN_{uc} (kN)** | | |
| | | | | | | | | Concrete compressive strength, f'_c | | |
| | | | | | | | | 20 MPa | 32 MPa | 40 MPa |
| 13 | 13 | 14 | 40 | 5 | 80 | 160 | 16.4 | 9.6 | 12.1 | 13.5 |
| | | | 75 | | | | 30.8 | 17.9 | 22.7 | 25.3 |
| | | | 110 | | | | 32.0 | 26.3 | 33.2 | 37.2 |
| 16 | 16 | 19 | 50 | 5 | 100 | 200 | 28.9 | 14.7 | 18.6 | 20.8 |
| | | | 70 | | | | 40.3 | 20.6 | 26.0 | 29.1 |
| | | | 90 | | | | 51.8 | 26.5 | 33.5 | 37.4 |
| 19 | 19 | 21 | 57 | 5 | 120 | 230 | 40.3 | 19.9 | 25.2 | 28.2 |
| | | | 80 | | | | 56.6 | 27.9 | 35.3 | 39.5 |
| | | | 90 | | | | 63.6 | 31.4 | 39.8 | 44.5 |

* Note: For shear loads acting towards an edge or where these optimum dimensions are not achievable, please use the simplified strength limit state design process to verify capacity.

**Note: Reduced characteristic ultimate concrete tensile capacity = ϕN_{uc} where $\phi = 0.6$ and N_{uc} = Characteristic ultimate concrete tensile capacity.

For conversion to Working Load Limit MULTIPLY ϕN_{uc} x 0.55

All data relevant for Non-cracked concrete

DESCRIPTION AND PART NUMBERS

| Anchor size, d_b (mm) | Effective length, L_e (mm) | Part No. Zn |
|-------------------------|------------------------------|-------------|
| 13 | 59 | BAC08075 |
| | 84 | BAC08100 |
| 16 | 71 | BAC10090 |
| | 106 | BAC10125 |
| 19 | 93 | BAC12115 |

Effective depth, h (mm)
 $h = L_e - t$
 t = total thickness of material(s) being fixed

Substrate thickness, b_m (mm)
 $b_m = h + (5 \times d_h)$

Drilled hole depth, h_1 (mm)
 $h_1 = h + (3 \times d_h)$
 h = Effective depth

ENGINEERING PROPERTIES - Carbon Steel

| Anchor size, d_b (mm) | Bolt stress area, A_s (mm ²) | Bolt yield strength, f_y (MPa) | Bolt UTS, f_u (MPa) | Section modulus, Z (mm ³) |
|-------------------------|--|----------------------------------|-----------------------|---|
| 13 | 77.8 | 680 | 830 | 97.0 |
| 16 | 134.4 | 680 | 830 | 219.8 |
| 19 | 196.0 | 680 | 830 | 387.2 |

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STRENGTH LIMIT STATE DESIGN

STEP 1 Select anchor to be evaluated

Table 1a Indicative combined loading - interaction diagram

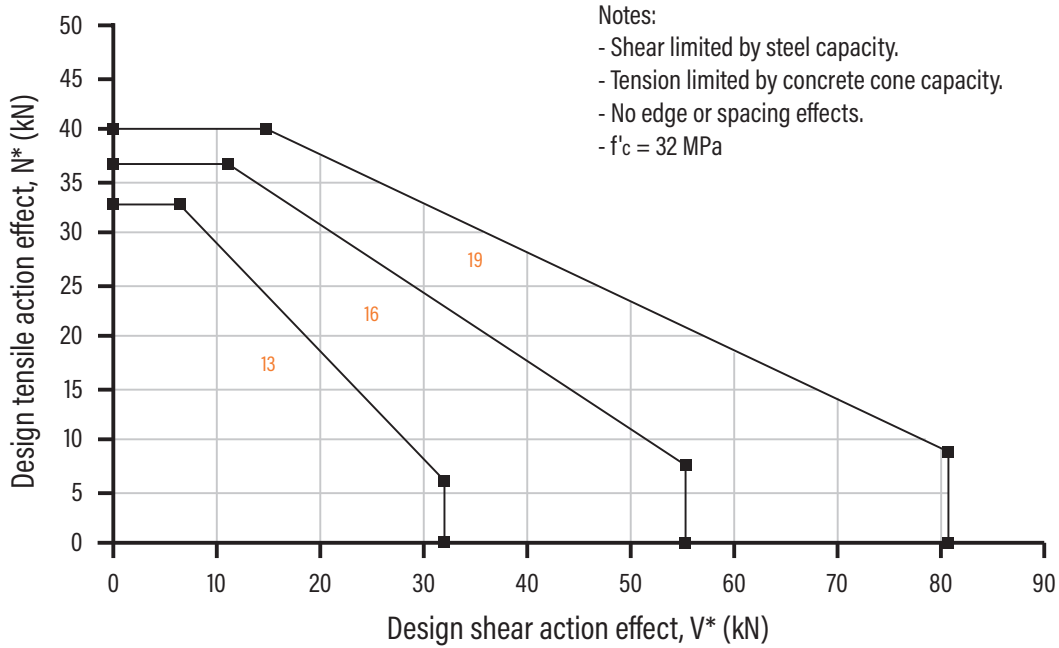


Table 1b Absolute minimum edge distance and anchor spacing values, e_m and a_m (mm)

| Anchor size, d_b (mm) | 13 | 16 | 19 | |
|-------------------------|----------------|-----|-----|-----|
| Edge distance, e_m | 65 | 80 | 95 | |
| Anchor spacing, a_m | $e \geq 6 d_b$ | 105 | 130 | 150 |
| | $e < 6 d_b$ | 130 | 160 | 190 |

Step 1c Calculate anchor effective depth, h (mm)

Refer to "Description and Part Numbers" table on the previous page.

Effective depth, h (mm)

$h = L_e - t$

t = total thickness of material(s) being fixed

Checkpoint 1 Anchor size determined, absolute minima compliance achieved, effective depth (h) calculated.

Boa™ Coil

STRENGTH LIMIT STATE DESIGN

Mechanical Anchoring

STEP 2 Verify concrete tensile capacity - per anchor

Table 2a Reduced characteristic ultimate concrete tensile capacity, ϕN_{uc} (kN), $\phi_c = 0.6$, $f'_c = 32$ MPa

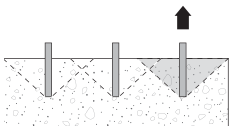
| Anchor size, d_b (mm) | 13 | 16 | 19 |
|---------------------------|------|------|------|
| Effective depth, h (mm) | | | |
| 40 | 12.1 | | |
| 45 | 13.6 | | |
| 50 | 15.1 | 18.6 | |
| 55 | 16.6 | 20.5 | |
| 60 | 18.1 | 22.3 | 26.5 |
| 70 | 21.2 | 26.0 | 30.9 |
| 80 | 24.2 | 29.8 | 35.3 |
| 90 | 27.2 | 33.5 | 39.8 |
| 100 | 30.2 | 37.2 | |
| 105 | 31.7 | | |
| 110 | 33.2 | | |

Note: Effective depth, h must be $\geq 3 \times$ anchor size, d_b in order to achieve tabled shear capacities.
All data relevant for Non-cracked concrete

Table 2b Concrete compressive strength effect, tension, X_{nc}

| f'_c (MPa) | 20 | 25 | 32 | 40 | 50 |
|--------------|------|------|------|------|------|
| X_{nc} | 0.79 | 0.88 | 1.00 | 1.12 | 1.25 |

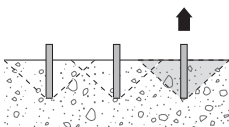
Table 2c Edge distance effect, tension, X_{ne}



| Anchor size, d_b (mm) | 13 | 16 | 19 |
|-------------------------|------|------|------|
| Edge distance, e (mm) | | | |
| 70 | 0.93 | | |
| 80 | 1.00 | 0.88 | |
| 90 | | 0.96 | |
| 100 | | 1.00 | 0.91 |
| 120 | | | 1.00 |

Table 2d Anchor spacing effect, end of a row, tension, X_{nae}

Note: For single anchor designs, $X_{nae} = 1.0$



| Anchor size, d_b (mm) | 13 | 16 | 19 |
|--------------------------|------|------|------|
| Anchor spacing, a (mm) | | | |
| 100 | 0.82 | | |
| 120 | 0.88 | | |
| 140 | 0.95 | 0.86 | |
| 160 | 1.00 | 0.92 | 0.85 |
| 180 | | 0.97 | 0.89 |
| 200 | | 1.00 | 0.94 |
| 220 | | | 0.98 |
| 230 | | | 1.00 |

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STRENGTH LIMIT STATE DESIGN

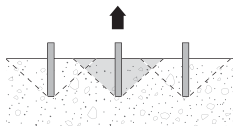


Table 2e Anchor spacing effect, internal to a row, tension, X_{nai}

Note: For single anchor designs, $X_{nai} = 1.0$

| Anchor size, d_b (mm) | 13 | 16 | 19 |
|--------------------------|------|------|------|
| Anchor spacing, a (mm) | | | |
| 100 | 0.64 | | |
| 120 | 0.77 | | |
| 140 | 0.90 | 0.73 | |
| 150 | 0.96 | 0.78 | 0.66 |
| 160 | 1.00 | 0.83 | 0.70 |
| 180 | | 0.94 | 0.79 |
| 200 | | 1.00 | 0.88 |
| 220 | | | 0.96 |
| 230 | | | 1.00 |

Checkpoint 2

Design reduced ultimate concrete tensile capacity, ϕN_{urc}

$$\phi N_{urc} = \phi N_{uc} * X_{nc} * X_{ne} * (X_{nae} \text{ or } X_{nai})$$

STEP 3

Verify anchor tensile capacity - per anchor

Table 3a Reduced characteristic ultimate steel tensile capacity, ϕN_{us} (kN), $\phi_n = 0.8$

| Anchor size, d_b (mm) | 13 | 16 | 19 |
|-------------------------|------|------|-------|
| Carbon steel | 51.7 | 89.2 | 130.1 |

Checkpoint 3

Design reduced ultimate tensile capacity, ϕN_{ur}

$$\phi N_{ur} = \text{minimum of } \phi N_{urc}, \phi N_{us}$$

Check $N^* / \phi N_{ur} \leq 1$,

if not satisfied return to step 1

Tensile performance conversion table

| Performance Required | Concrete Tensile Performance | | Steel Tensile Performance | |
|----------------------|------------------------------|-------------------------------------|---------------------------|------------------------------------|
| | Notation | Concrete Tension Capacity | Notation | Carbon Steel Tension Capacity |
| Strength Limit State | ϕN_{urc} | MULTIPLY $\phi N_{urc} \times 1.00$ | ϕN_{us} | MULTIPLY $\phi N_{us} \times 1.00$ |
| Working Load Limit | N_{ac} | MULTIPLY $\phi N_{urc} \times 0.55$ | N_{as} | MULTIPLY $\phi N_{us} \times 0.56$ |
| Cyclic Loading | N_{yc} | MULTIPLY $\phi N_{urc} \times 0.55$ | N_{ys} | MULTIPLY $\phi N_{us} \times 0.56$ |
| Fire Resistance | $N_{Rk,c,fit}$ | Refer to Fire Rated Anchors | $N_{Rk,s,fit}$ | Refer to Fire Rated Anchors |
| Seismic | $N_{Rd,p,sis}^0$ | Refer to Seismic Anchors | $N_{Rd,s,sis}$ | Refer to Seismic Anchors |

NOTE: Design Tensile Capacity is the minimum of Concrete Tension and Steel Tension Capacities

Boa™ Coil

STRENGTH LIMIT STATE DESIGN

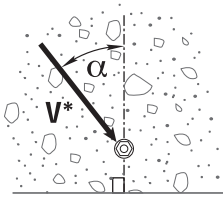
Mechanical Anchoring

STEP 4 Verify concrete shear capacity - per anchor

Table 4a Reduced characteristic ultimate concrete edge shear capacity, ϕV_{uc} (kN), $\phi_c = 0.6$, $f_c = 32$ MPa

| Anchor size, d_b (mm) | 13 | 16 | 19 |
|-------------------------|-------|-------|-------|
| Edge distance, e (mm) | | | |
| 70 | 8.7 | | |
| 80 | 10.7 | 11.9 | |
| 100 | 14.9 | 16.6 | 18.0 |
| 150 | 27.4 | 30.4 | 33.2 |
| 200 | 42.2 | 46.8 | 51.1 |
| 250 | 59.0 | 65.5 | 71.3 |
| 300 | 77.6 | 86.1 | 93.8 |
| 400 | 119.4 | 132.5 | 144.4 |
| 500 | 166.9 | 185.2 | 201.8 |
| 600 | 219.4 | 243.4 | 265.3 |

Note: Effective depth, h must be $\geq 3 \times$ anchor size, d_b in order to achieve tabled shear capacities.
All data relevant for Non-cracked concrete



Load direction effect, conc. edge shear, X_{vd}

Table 4b Concrete compressive strength effect, concrete edge shear, X_{vc}

| f_c (MPa) | 20 | 25 | 32 | 40 | 50 |
|-------------|------|------|------|------|------|
| X_{vc} | 0.79 | 0.88 | 1.00 | 1.12 | 1.25 |

Table 4c Load direction effect, concrete edge shear, X_{vd}

| Angle, α° | 0 | 10 | 20 | 30 | 40 | 50 | 60 | 70 | 80 | 90 - 180 |
|-----------------------|------|------|------|------|------|------|------|------|------|----------|
| X_{vd} | 1.00 | 1.04 | 1.16 | 1.32 | 1.50 | 1.66 | 1.80 | 1.91 | 1.98 | 2.00 |

Table 4d Anchor spacing effect, concrete edge shear, X_{va}

Note: For single anchor designs, $X_{va} = 1.0$

| Edge distance, e (mm) | 35 | 50 | 70 | 80 | 100 | 150 | 200 | 250 | 300 | 400 | 500 | 600 |
|--------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Anchor spacing, a (mm) | | | | | | | | | | | | |
| 50 | 0.79 | 0.70 | 0.64 | 0.63 | 0.60 | 0.57 | 0.55 | 0.54 | 0.53 | 0.53 | 0.52 | 0.52 |
| 75 | 0.93 | 0.80 | 0.71 | 0.69 | 0.65 | 0.60 | 0.58 | 0.56 | 0.55 | 0.54 | 0.53 | 0.53 |
| 100 | 1.00 | 0.90 | 0.79 | 0.75 | 0.70 | 0.63 | 0.60 | 0.58 | 0.57 | 0.55 | 0.54 | 0.53 |
| 125 | | 1.00 | 0.86 | 0.81 | 0.75 | 0.67 | 0.63 | 0.60 | 0.58 | 0.56 | 0.55 | 0.54 |
| 150 | | | 0.93 | 0.88 | 0.80 | 0.70 | 0.65 | 0.62 | 0.60 | 0.58 | 0.56 | 0.55 |
| 175 | | | 1.00 | 0.94 | 0.85 | 0.73 | 0.68 | 0.64 | 0.62 | 0.59 | 0.57 | 0.56 |
| 200 | | | | 1.00 | 0.90 | 0.77 | 0.70 | 0.66 | 0.63 | 0.60 | 0.58 | 0.57 |
| 225 | | | | | 0.95 | 0.80 | 0.73 | 0.68 | 0.65 | 0.61 | 0.59 | 0.58 |
| 250 | | | | | 1.00 | 0.83 | 0.75 | 0.70 | 0.67 | 0.63 | 0.60 | 0.58 |
| 275 | | | | | | 0.87 | 0.78 | 0.72 | 0.68 | 0.64 | 0.61 | 0.59 |
| 300 | | | | | | 0.90 | 0.80 | 0.74 | 0.70 | 0.65 | 0.62 | 0.60 |
| 400 | | | | | | 1.00 | 0.90 | 0.82 | 0.77 | 0.70 | 0.66 | 0.63 |
| 500 | | | | | | | 1.00 | 0.90 | 0.83 | 0.75 | 0.70 | 0.67 |
| 750 | | | | | | | | 1.00 | 1.00 | 0.88 | 0.80 | 0.75 |
| 1000 | | | | | | | | | | 1.00 | 0.90 | 0.83 |
| 1250 | | | | | | | | | | | 1.00 | 0.92 |
| 1500 | | | | | | | | | | | | 1.00 |

Boa™ Coil

STRENGTH LIMIT STATE DESIGN

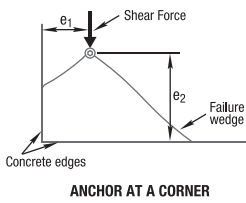
Table 4e Multiple anchors effect, concrete edge shear, X_{vn}

Note: For single anchor designs, $X_{vn} = 1.0$

| Anchor spacing / Edge distance, a / e | 0.20 | 0.40 | 0.60 | 0.80 | 1.00 | 1.20 | 1.40 | 1.60 | 1.80 | 2.00 | 2.25 | 2.50 |
|---------------------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Number of anchors, n | | | | | | | | | | | | |
| 2 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| 3 | 0.72 | 0.76 | 0.80 | 0.83 | 0.86 | 0.88 | 0.91 | 0.93 | 0.95 | 0.96 | 0.98 | 1.00 |
| 4 | 0.57 | 0.64 | 0.69 | 0.74 | 0.79 | 0.82 | 0.86 | 0.89 | 0.92 | 0.94 | 0.97 | 1.00 |
| 5 | 0.49 | 0.57 | 0.63 | 0.69 | 0.74 | 0.79 | 0.83 | 0.87 | 0.90 | 0.93 | 0.97 | 1.00 |
| 6 | 0.43 | 0.52 | 0.59 | 0.66 | 0.71 | 0.77 | 0.81 | 0.85 | 0.89 | 0.93 | 0.96 | 1.00 |
| 7 | 0.39 | 0.48 | 0.56 | 0.63 | 0.69 | 0.75 | 0.80 | 0.84 | 0.88 | 0.92 | 0.96 | 1.00 |
| 8 | 0.36 | 0.46 | 0.54 | 0.61 | 0.68 | 0.74 | 0.79 | 0.84 | 0.88 | 0.92 | 0.96 | 1.00 |
| 9 | 0.34 | 0.44 | 0.52 | 0.60 | 0.67 | 0.73 | 0.78 | 0.83 | 0.87 | 0.91 | 0.96 | 1.00 |
| 10 | 0.32 | 0.42 | 0.51 | 0.59 | 0.66 | 0.72 | 0.77 | 0.82 | 0.87 | 0.91 | 0.96 | 1.00 |
| 15 | 0.26 | 0.37 | 0.47 | 0.55 | 0.63 | 0.70 | 0.76 | 0.81 | 0.86 | 0.90 | 0.95 | 1.00 |
| 20 | 0.23 | 0.35 | 0.45 | 0.54 | 0.61 | 0.68 | 0.75 | 0.80 | 0.85 | 0.90 | 0.95 | 1.00 |

Table 4f Anchor at a corner effect, concrete edge shear, X_{vs}

Note: For $e_1/e_2 > 1.25$, $X_{vs} = 1.0$



| Edge distance, e_2 (mm) | 25 | 30 | 35 | 50 | 60 | 75 | 125 | 200 | 300 | 400 | 600 | 900 |
|---------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Edge distance, e_1 (mm) | | | | | | | | | | | | |
| 25 | 0.86 | 0.77 | 0.70 | 0.58 | 0.53 | 0.49 | 0.41 | 0.37 | 0.35 | 0.34 | 0.32 | 0.32 |
| 30 | 0.97 | 0.86 | 0.78 | 0.64 | 0.58 | 0.52 | 0.43 | 0.38 | 0.36 | 0.34 | 0.33 | 0.32 |
| 35 | 1.00 | 0.95 | 0.86 | 0.69 | 0.63 | 0.56 | 0.46 | 0.40 | 0.37 | 0.35 | 0.33 | 0.32 |
| 50 | 1.00 | 1.00 | 1.00 | 0.86 | 0.77 | 0.67 | 0.52 | 0.44 | 0.39 | 0.37 | 0.35 | 0.33 |
| 60 | 1.00 | 1.00 | 1.00 | 0.97 | 0.86 | 0.75 | 0.57 | 0.47 | 0.41 | 0.38 | 0.36 | 0.34 |
| 75 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 | 0.64 | 0.51 | 0.44 | 0.41 | 0.37 | 0.35 |
| 125 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 | 0.65 | 0.53 | 0.48 | 0.42 | 0.38 |
| 200 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 | 0.67 | 0.58 | 0.49 | 0.42 |
| 300 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 | 0.72 | 0.58 | 0.49 |
| 400 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 | 0.67 | 0.55 |
| 500 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.77 | 0.61 |
| 600 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 | 0.67 |
| 900 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.86 |

Checkpoint 4

Design reduced ultimate concrete edge shear capacity, ϕV_{urc}

$$\phi V_{urc} = \phi V_{uc} * X_{vc} * X_{vd} * X_{va} * X_{vn} * X_{vs}$$

STEP 5

Verify anchor shear capacity - per anchor

Table 5a Reduced characteristic ultimate steel shear capacity, ϕV_{us} (kN), $\phi_v = 0.8$

| Anchor size, d_b (mm) | 13 | 16 | 19 |
|-------------------------|------|------|------|
| $h \geq 6 \times d_b$ | 32.0 | 55.3 | 80.7 |
| $h \geq 5 \times d_b$ | 26.7 | 46.1 | 67.2 |
| $h \geq 4 \times d_b$ | 21.3 | 36.9 | 53.8 |
| $h \geq 3 \times d_b$ | 16.0 | 27.7 | 40.3 |

Boa™ Coil

STRENGTH LIMIT STATE DESIGN

Mechanical Anchoring

Checkpoint 5

Design reduced ultimate shear capacity, ϕV_{ur}
 $\phi V_{ur} = \text{minimum of } \phi V_{urc}, \phi V_{us}$
 Check $V^* / \phi V_{ur} \leq 1$,
 if not satisfied return to step 1

Shear performance conversion table

| Performance Required | Concrete Shear Performance | | Steel Shear Performance | |
|----------------------|----------------------------|------------------------------------|-------------------------|------------------------------------|
| | Notation | Concrete Shear Capacity | Notation | Carbon Steel Shear Capacity |
| Strength Limit State | ϕV_{uc} | MULTIPLY $\phi V_{uc} \times 1.00$ | ϕV_{us} | MULTIPLY $\phi V_{us} \times 1.00$ |
| Working Load Limit | V_{ac} | MULTIPLY $\phi V_{uc} \times 0.55$ | V_{as} | MULTIPLY $\phi V_{us} \times 0.50$ |
| Cyclic Loading | V_{yc} | MULTIPLY $\phi V_{uc} \times 0.55$ | V_{ys} | MULTIPLY $\phi V_{us} \times 0.50$ |
| Fire Resistance | $V_{Rk,c,fit}$ | Refer to Fire Rated Anchors | $V_{Rk,s,fit}$ | Refer to Fire Rated Anchors |
| Seismic | $V_{Rd,c,sis}^0$ | Refer to Seismic Anchors | $V_{Rd,s,sis}^0$ | Refer to Seismic Anchors |

NOTE: Design Shear Capacity is the minimum of Concrete Shear and Steel Shear Capacities

STEP 6

Combined loading and specification

Checkpoint 6

Check
 $N^* / \phi N_{ur} + V^* / \phi V_{ur} \leq 1.2$,
 if not satisfied return to step 1

Specify
 Ramset Boa™ Coil Anchor,
 (Anchor Size) ((Part Number)).
 Maximum fixed thickness to be (t) mm.

Example
 Ramset Boa™ Coil Anchor,
 16 mm (BAC10125).
 Maximum fixed thickness to be 14 mm.
 To be installed in accordance to Ramset™
 Installation Instructions.